

# Coastal Engineering Technical Note



### MOVEMENT AND STATIC STRESSES IN DOLOSSE

- 1. In 1986 the seaward end of the mainstem of the outer breakwater at Crescent City, CA, was rehabilitated using 680 fiber reinforced, 42-ton dolosse. As part of the Crescent City Prototype Dolosse Study (CCPDS), twenty of the 680 dolosse were instrumented to measure loading on the armor units. The instrumented units were placed near the center of the rehabilitated area. Four of the dolosse were placed in the bottom layer and the remaining 16 were positioned in the top layer. During the two years following the repair, a wealth of incident wave conditions and dolos loading and movement data were collected. Details of the CCPDS are presented in Howell, et al. (in publication).
- 2. Strain gages were positioned on rebar rosettes inside the instrumented dolosse in such a manner that two moments and a torque could be measured at the fluke-shank interface on one end of the dolosse, Figure 1 (Howell 1988). For the CCPDS, Burcharth and Howell (1988) have proposed a concrete failure criteria for use in dolos design. The criteria compares maximum principal tensile stress to the concrete rupture strength. Melby (1989) discusses the calculation of the maximum principal tensile stress for a cross-section in the dolos shank, and limitations of this methodology, indicating that it can be used with confidence to calculate maximum principal stress in dolos armor units.
- 3. Pulsating waves, impacts between units and static loads make up the three principal categories of loadings on concrete armor units (Burcharth 1984). During the CCPDS, no impact loads were observed. The maximum principal stress data recorded were comprised of pulsating wave loads and static loads induced by self weight and loads from adjacent units which include interlocking and wedging effects. That portion of the maximum principal stress associated with pulsating short and long period loadings can, in principle, be separated from the total stress data. The average of a 20 min data signal is used to represent the mean. The mean is the static response. The signal minus the mean is the pulsating response. Burcharth, et al. (1991) describes this analysis technique in detail.
- 4. Another important element of CCPDS was the collection of cumulative dolos movement data. This was accomplished through the use of ground-truthed photogrammetric survey techniques described by Kendall (1988). Eighteen dolosse located in the instrumented dolos area and 8 additional uninstrumented dolosse located in the 1986 rehabilitation area were targeted so movement could be closely monitored and documented. The targeted instrumented and uninstrumented dolosse are indicated by alpha and numeric characters, respectively, in Figure 2.

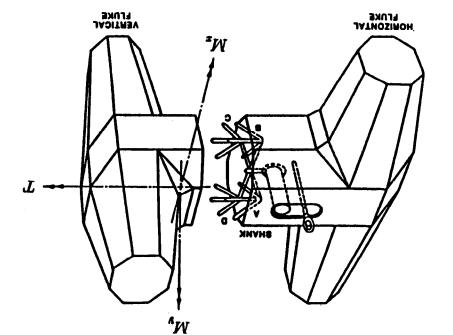
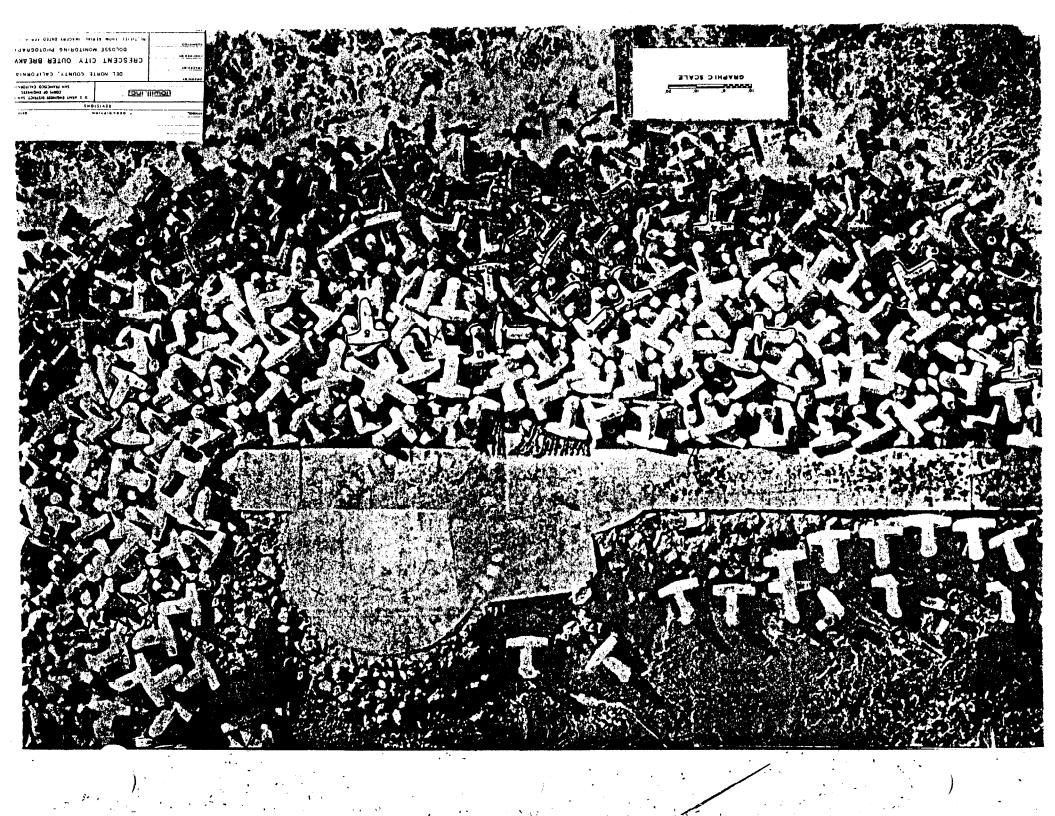


Figure 1. Prototype Internal Dolos Instrumentation and Measurement Definition

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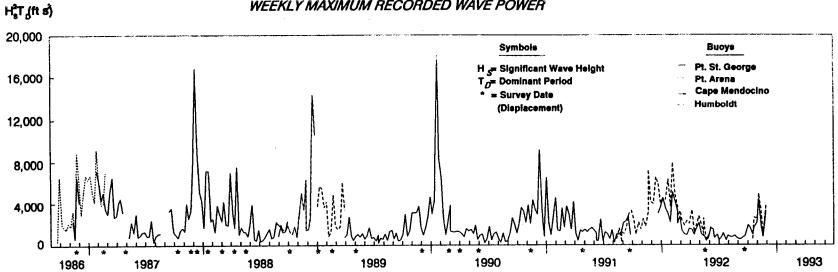
- 5. At the close of the CCPDS, following the post construction nesting of the dolosse, Kendall and Melby (1989) reported the dolosse cumulative average movement was leveling off, but some consolidation was still occurring in response to storm wave conditions. A set of static loading data collected in July 1989 showed that the rate of increase in static loads had decreased, but, overall average static loads were still increasing.
- 6. The concrete used in the Crescent City dolosse had a 28 day flexural tensile strength of 984 psi (Kendall and Melby, 1989). Static data collected up through July 1989 for Crescent City dolosse indicated tensile stress magnitudes as high as 525 psi, Table 1. (Note that the stresses in Table 1 are based on a 4-day moving window average and that data from dolosse A and 0 were assumed unreliable.) This meant that in some of the dolosse close to one half of the dolos tensile strength was being used to resist static loads. Howell, Rhee and Rosati (1989) have shown that pulsating wave loads for the Crescent City Breakwater could result in maximum principal stresses as high as 70 psi. Subsequent laboratory work by Markle (1989 and in publication) revealed maximum principal stresses of up to 110 psi due to pulsating wave loadings. When the probable maximum pulsating stresses, as measured in the prototype and recreated in the model, were added to measured prototype static stresses, well over half of the tensile strength was being used to resist load induced stresses.
- 7. As a result of the items discussed in paragraphs 5 and 6, it was determined in FY 89 that a low level dolos monitoring effort needed to be continued at Crescent City to define and document the continued cumulative dolos motions and changing static loads. Under the Monitoring Completed Coastal Projects Program, a work unit entitled Periodic Inspections had just begun and Crescent City has been included in this effort through September 1993. Results of photogrammetric surveys through September 1992 show that dolos movement is still continuing, but the magnitude of recently measured movement is very low, Figure 3.
- 8. In July 1990, the seven instrumented dolosse still working at Crescent City were monitored for one week to record static stresses. One of these dolosse had a bad channel which made it impossible to resolve principal stress for that dolosse. Data from the six functional dolosse showed that there had been a significant increase in static stresses in 5 of the 6 dolosse (near bottom of Table 1). Note the 500 psi static stress level in dolos M and the 400 psi static stress levels in dolosse E, P, and C. When the probable pulsating or wave loading stresses are added to these static stress levels, there is little or no reserve strength to resist possible impact induced stresses or increased static stresses. The increase in static stress between 1989 and 1990 is reflected in the overall average static stress level. The average increase during this year was 32%.
- 9. Static stresses in the six functional dolosse were again measured for one day in June 1991. These data again showed increases in stress for 5 of the 6 dolosse with the associated overall average static stress level increasing again by 8%.

Table 1. Average Static Stresses in Crescent City Instrumented Dolosse.

	time in	dd03°	ldd04	DD05	dd06	dd07	dd08	dd09	dd10	dd12	dd13	dd14	dd16		dd19
	days from	E	V	G	F	Α	D	N	T	P	С	K	M	Н	0
Date	9/1/86	<del>                                     </del>													
							ļ				<b></b>	ļ	<del> </del>	<b> </b>	
14-Feb-87	167					343				ļ	L	525	244	ļ	886
26-Feb-87	179		77	<u> </u>	333	359		295	<b></b>		ļ		244		892
05-Mar-87	186		75		294	374	106	309		255		392			879
13-Mar-87	194		72	177	286	390	71	321	331	242		259 237	204 185		86
17-Mar-87	198		73	179	283			333	332	<b> </b>	95			299	89
23-Mar-87	204		75		279		<b></b>	315		<b></b>	95	243			
28-Mar-87	209			171	273			371	[	<b></b>		<u> </u>	204	290	
04-Apr-87	216			164	274			325		ļ		ļ	228 237		819 81
10-Apr-87	222			161	281					ļ	ļ	ļ	237	<del> </del>	81
27-Jun-87	300			i				ļ		ļ	ļ			202	<del></del>
21-Nov-87	447				<u> </u>	357		318		252		<b> </b>	296		ļ
24-Nov-87	450					348		340	<u></u>	249		Ĺ	284		<b></b>
30-Nov-87	456			<u> </u>	299			361	<u> </u>	247	110		294		<b></b>
07-Dec-87	463	277	7		299	499		383	<u> </u>	264	122		280		
13-Dec-87	469					658		404	<u> </u>	277	135		267	354	<u> </u>
16-Dec-87	472	325	5	130		680		426	İ	292	138		254	363	
22-Dec-87	478	285	5	128		_676		448		288	141	<u> </u>	267		
30-Dec-87	486	305	5	132		705		457		300	142		269		
05-Jan-88	492	323	3	137			130	449		284	147		261	383	
10-Jan-88	497	311		142			126	460		305	142		278		
17-Jan-88				130			135	469		324	148		283		
30-Jan-88	517	351		129			144	460		346	130		277		
18-Feb-88	536			146			146	467		342	148		270		
04-Mar-88			1	145			152	471		333	135		275	362	
19-Sep-88			<del>                                     </del>											<u> </u>	
21-Jul-89			155	124	1	258	171	T		337			385		
22-Jul-89	1056					258	171			337		<u> </u>	386	<u> </u>	
13-Dec-89	1200		1		<del>                                     </del>								L		
10-Jul-90			176	101						392	430		496		
16-Jul-90				92	<del> </del>	<u> </u>	1			385			489		<u> </u>
03-Jun-91	1737					1				438	412		517	1	
19-Sep-91	1814			173		<del> </del>	1	1		413	378	3	453	3	
21-Jul-92		439	184	195		1	<b> </b>	1		410	378**		453**		
	E NOT FUN		PREVIOUS	SAMPLING	VALUES HA	WE BEEN F	OLLED FO	WARD.	1						

<sup>\*</sup>EACH COLUMN PRESENTS STATIC STRESS TIME HISTORY FOR ONE DOLOS( Example: dd03 and E are symbols for one dolos).

# WEEKLY MAXIMUM RECORDED WAVE POWER



# AVERAGE CUMULATIVE DOLOS DISPLACEMENT VERSUS TIME

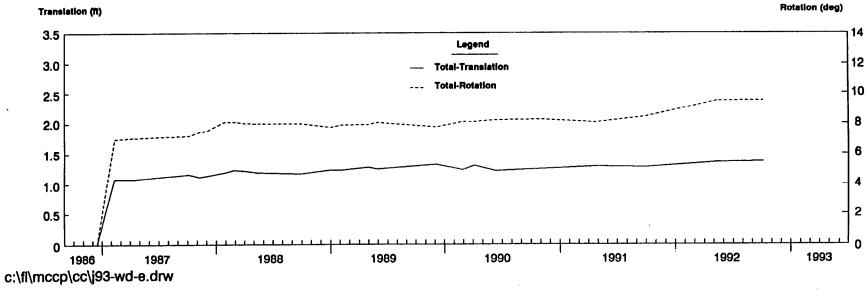


Figure 3. Survey dates correlated with wave power and cumulative dolos translation and rotation.

- 10. During the period 13 July 1991 through 19 August 1991 four earthquakes were reported along the northern California coast with epicenters approximately 65 miles west of Crescent City and magnitudes from 5.8 to 6.9 on the Richter scale. A team of CERC personnel visited the Crescent City breakwater in September 1991 and collected static stress data for a one day period. According to several Crescent City locals, little ground movement was noted in the Crescent City area during these earthquakes and that a seasonal storm with approximately 20 ft waves had hit the breakwater subsequent to the earthquakes. The data in Table 1 show that the average static stress had decreased in four of the six working dolosse since June 1991.
- 11. Static stress data were collected on 21 July 1992. Four dolosse were found to have all data channels still working, which allowed calculation of maximum principal static stress. The average static stress in these four dolosse showed an 8% increase over the average stress measured in September 1991. Other units have part of the data channels working, which continue to provide good data on existing strain, but will not allow calculation of principal, static stress levels.
- 12. Figure 4 presents, for each four day window of data, the average static stress for all reliable, working dolosse plotted against time. The best fit straight line through these data reveals an 11% increase in static stress per year.
- In summary, before the CCPDS, it was postulated that dolosse go through a post construction nesting period and then movement tends to slow down. The CCPDS and subsequent MCCP data confirm this. Prior to CCPDS, it was also felt that dolosse breakage was largely due to combined impact loads between dolosse and pulsating wave loadings which over stressed the dolosse and that static loads did not play a significant role. The CCPDS and MCCP data show that for larger sized dolosse, static stresses can use up a significant portion of a dolos's tensile strength and that the added pulsating loads in some cases could push the maximum principal stress levels near the postulated Crescent City 700 psi design tensile limit (Kendall and Melby Continued monitoring of Crescent City will aid in understanding and defining post construction movement and static stress responses of dolosse. The information obtained in this study is being incorporated into the dolos design procedure developed for Crescent City, and it will also be used in the development of a general hydraulic and structural design procedure for concrete armor units (Melby, in preparation). The latter will be developed under work units in the Coastal Research and Development Program and the Repair, Evaluation, Maintenance and Rehabilitation Program II. These work units include additional mid- and large-scale model tests to develop an understanding of impact induced stresses and to gain more insight into combined hydraulic and structural stability of slender concrete armor units.
- 14. Based on current scopes of work in the "Periodic Inspections" work unit, static stress measurements will be made during the summer of 1993 and photogrammetric work will continue at least through the end of FY 93. These data are being used to define and correlate dolos movement and static stress levels over an eight year post construction period.

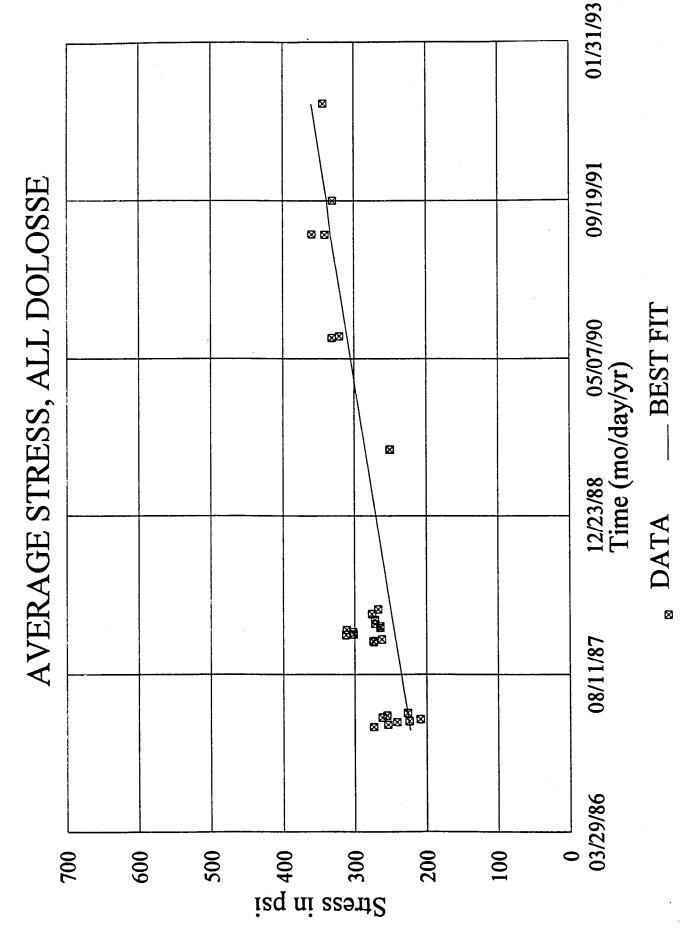


Figure 4. Average static str versus time for all dolosse data.

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15. This CETN will periodically be revised when additional static stress and movement data become available. If there are questions relative to the information contained herein, please contact Mr. Dennis G. Markle, CEWES-CW-P (commercial 601-634-3680 or FTS 542-3680) or Mr. Jeff Melby, CEWES-CW-R (commercial 601-634-2062 or FTS 542-2062).

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